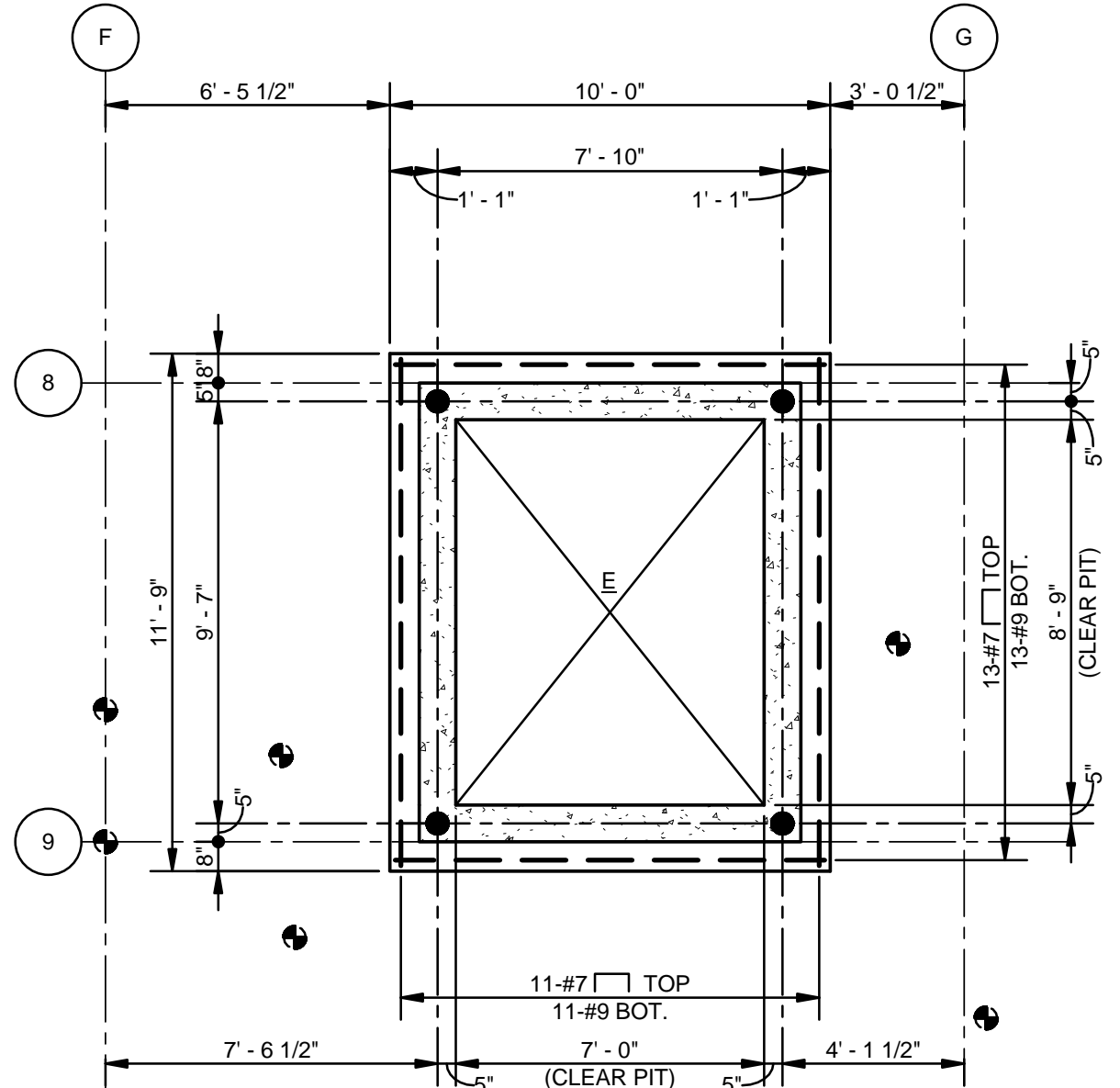


1 GENERAL NOTES

1. EXISTING CONDITIONS:
ALL DIMENSIONS AND CONDITIONS TYING INTO OR GOVERNED BY EXISTING CONSTRUCTION ARE APPROXIMATE AND ARE NOT PURPORTED TO BE CORRECT. ALL SUCH DIMENSIONS AND CONDITIONS SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO THE PREPARATION OF SHOP DRAWINGS. FIRST SUBMITTAL OF SHOP DRAWINGS MUST CONTAIN CORRECT CONDITIONS AND DIMENSIONS OBTAINED FROM THE FIELD. IF CONDITIONS AND DIMENSIONS VARY GREATLY FROM THOSE SHOWN, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT BEFORE PREPARATION OF SHOP DRAWINGS.
2. EXISTING UTILITIES:
IT IS THE CONTRACTOR'S RESPONSIBILITY TO LOCATE ALL EXISTING UTILITIES PRIOR TO ANY CONSTRUCTION OR FABRICATION. ANY CONFLICTS OR POTENTIAL CONFLICTS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE ARCHITECT.
3. SHORING:
SHORE AND BRACE ALL EXISTING FRAMING AS REQUIRED IN ORDER TO ACCOMPLISH WORK SHOWN ON DRAWINGS.
4. DRILLING HOLES FOR ANCHORS AND CORING HOLES IN EXISTING CONCRETE:
A. PRIOR TO DRILLING OR CORING HOLES, THE CONTRACTOR SHALL LOCATE EXISTING REINFORCING STEEL, POST-TENSIONING, CONDUIT, PIPING, ETC. IN THE AREA WHERE NEW HOLES ARE TO BE INSTALLED THROUGH NON-DESTRUCTIVE TESTING SUCH AS WITH AN X-RAY, RADAR, OR WITH OTHER NON-DESTRUCTIVE DEVICES.
B. MARK THE LOCATION OF ALL REINFORCING STEEL, POST-TENSIONING, CONDUIT, PIPING, ETC. ON THE EXISTING INTERFERENCES ON THE SURFACE OF THE SLAB.
C. IF NEW HOLE LOCATIONS CONFLICT WITH EXISTING REINFORCING, POST-TENSIONING, CONDUIT, PIPING, ETC., THE CONTRACTOR SHALL NOTIFY THE ENGINEER BEFORE INSTALLING THE NEW HOLES.
D. VERIFY NO CONFLICTS EXIST AT NEW HOLE LOCATIONS BY SMALL DRILLED PILOT HOLES. IF NO CONFLICTS EXIST, COMPLETE THE INSTALLATION. IN THE CASE OF STEEL TO BE FASTENED TO THE CONCRETE WITH MULTIPLE ANCHORS, FABRICATE, FROM A FIELD TEMPLATE, THE STEEL TO BE FASTENED TO THE CONCRETE BY THE ANCHORS AND COMPLETE THE INSTALLATION.
E. WHEN INSTALLING NEW HOLES, CARE SHALL BE EXERCISED SO AS NOT TO NICK OR CUT EXISTING REINFORCING STEEL, POST-TENSIONING, CONDUIT, PIPING, ETC.
5. DAMAGE TO EXISTING CONSTRUCTION WHICH WILL NOT DAMAGE ADJACENT EXISTING CONSTRUCTION WHICH IS TO REMAIN.
6. VAPOR BARRIER:
A. UNLESS OTHERWISE STATED IN SPECIFICATIONS, PROVIDE OPAQUE, 15 MIL POLYETHYLENE VAPOR BARRIER CONFORMING TO ASTM E1745 ON TOP OF FILL UNDER ALL INTERIOR SLABS AND BEAMS. OMIT ON TOP OF PILES.
B. FOLLOW MANUFACTURER'S RECOMMENDATIONS AND USE MANUFACTURER'S RECOMMENDED ADHESIVE AND PRESSURE-SENSITIVE TAPE FOR SEALING HOLES AND JOINTS IN VAPOR BARRIER.
C. SHALL BE NEATLY PLACED, FOLLOW THE PROFILE OF THE BOTTOM OF THE SLAB AND BEAMS, AND BE IN INTIMATE CONTACT WITH THE FILL.
7. GROUTED HELICAL PILES:
A. MANUFACTURER: MAGNUM GEO-SOLUTIONS
B. MANUFACTURED COMPONENTS: HELIX PLATE, SHAFT, STEEL BRACKET
C. GROUT: AS SPECIFIED BY MANUFACTURER
D. COATING: THE TOP 8 FEET (MIN.) OF SHAFT SHALL BE HOT DIP GALVANIZED.
E. INSTALLATION: HELICAL PILES TO BE INSTALLED BY CERTIFIED CONTRACTOR, CONNECTION TO STRUCTURE, TORQUE POWER UNITS, ADAPTERS, EXTENSIONS, TORQUE RECORDINGS, PROPER ALIGNMENT, REVOLUTIONS PER MINUTE, CROWD FORCE, COUPLING BOLTS TO BE IN COMPLIANCE WITH MANUFACTURER'S RECOMMENDATIONS.
F. FIELD QUALITY CONTROL: INSTALLATION, FIELD TESTING, GROUT INTAKE & TORQUE MONITORING SHALL BE RECORDED BY A QUALIFIED TESTING LAB. PILE SHALL BE MARKED IN ONE FOOT INCREMENTS AND TORQUE READINGS TAKEN AT EACH INCREMENT.
G. LOAD TEST: ONE REQUIRED
H. DESIGN LOAD: 22 TONS (FACTOR OF SAFETY: 2)
J. DESIGN: DESIGN, SIZE, DEPTH & TYPE OF HELICAL PILE TO BE CONDUCTED BY PILE MANUFACTURER. PILE DESIGN CALCULATIONS AND DRAWINGS SHALL BE STAMPED AND SIGNED BY AN ENGINEER LICENSED IN LOUISIANA AND SUBMITTED TO ARCHITECT FOR REVIEW.
8. CONCRETE:
A. ACI 301-10 SPECIFICATIONS
B. NORMAL WEIGHT CONCRETE: 150 PCF
C. LIGHTWEIGHT CONCRETE: 115 PCF
D. COMPRESSIVE STRENGTH AT 28 DAYS:
4,000 PSI, EXCEPT CONCRETE ON METAL DECK: 3,000 PSI
9. REINFORCING STEEL:
A. BARS: ASTM A615, GRADE 60
B. WELDED WIRE MESH: ASTM A1064
10. REINFORCING CLEARANCES REQUIRED ARE AS FOLLOWS:
ACI 117 STANDARDS. UNLESS SPECIFICALLY NOTED OR SHOWN ON THE DRAWINGS, REINFORCING CLEARANCES SHALL BE AS FOLLOWS:
A. SLABS: 3/4" CLEAR TOP & BOTTOM FORMED.
1" CLEAR BOTTOM, 3/4" CLEAR TOP ON GRADE.
B. BEAMS: 1 1/2" CLEAR BOTTOM FORMED, 3" CLEAR BOTTOM CAST ON EARTH, 1 1/2" CLEAR SIDES AND TOP FORMED, 3" CLEAR SIDES EARTH FORMED, 1 1/2" CLEAR TOP.
C. COLUMNS: 1 1/2" CLEAR, TYPICAL.
D. WALLS: 1 1/2" CLEAR, TYPICAL.
11. REINFORCING DETAILS FOR STRUCTURALLY SUPPORTED CONCRETE:
ACI 315 STANDARDS. UNLESS SPECIFICALLY NOTED OR SHOWN ON THE DRAWINGS, BAR LAPS AND CONFIGURATIONS SHALL BE AS FOLLOWS:
A. TOP BARS: HOOK AT NON-CONTINUOUS ENDS. LAP 30 DIAS.
AT MID-SPAN, FOR CONTINUOUS REINFORCING NOTED ON THE CONSTRUCTION DOCUMENTS AS FULLY CONT., SEE NOTE "F".
B. BOTTOM BARS: LAP 6" AT CENTER OF SUPPORT.
C. TEMPERATURE BARS IN SLAB AND INTERMEDIATE HORIZONTAL BARS IN WALLS AND BEAMS: TENSION LAP SPLICES, SEE TABLE BELOW.
D. SLAB TOP REINFORCING SUPPORT BARS: SLAB TOP REINFORCING BARS LESS THAN 6 FEET IN LENGTH SHALL HAVE 2-#4 CONT. SUPPORT BARS AND SLAB TOP REINFORCING BARS GREATER THAN 6 FEET IN LENGTH SHALL HAVE #4 SUPPORT BARS EQUALLY SPACED AT NO MORE THAN 4'-0" O.C.
E. CORNER BARS: PROVIDE CORNER BARS AT EACH OUTSIDE CORNER FOR EACH HORIZONTAL BAR IN WALLS AND BEAMS, CORNER BARS SHALL LAP WITH HORIZONTAL BARS. PROVIDE "U" BARS AT WALL ENDS. LAP #3 TO #6 BARS EACH WAY AND LAP #7 TO #11 BARS 48" EACH WAY. HOOK INCH BARS IN WALLS AT ENDS.
F. LAP SPLICE TABLE FOR FULLY CONTINUOUS REINFORCING:
- | LAP SPLICES (INCHES) | | |
|----------------------|----------|----------------------------------|
| LOCATION | BAR SIZE | |
| SLABS | TOP | #3 15 24 36 48 78 96 117 #10 #11 |
| | OTHER | 12 19 28 37 60 74 90 140 165 |
| OTHER | TOP | 24 32 40 48 70 80 91 108 122 |
| | OTHER | 19 25 31 37 54 62 70 102 113 |
12. CONDUITS AND PIPES EMBEDDED IN CONCRETE:
A. CONDUITS, PIPES, AND SLEEVES OF ANY MATERIAL, NOT HARMFUL TO CONCRETE SHALL BE PERMITTED TO BE EMBEDDED IN CONCRETE WITH APPROVAL OF THE ENGINEER. THE REGULATIONS ARE FOLLOWED AS OUTLINED IN THE APPLICABLE ACI CODES.
B. CONDUITS, PIPES, AND SLEEVES PASSING THROUGH A SLAB OR BEAM SHALL NOT SIGNIFICANTLY IMPAIR THE STRENGTH OF CONSTRUCTION AS DETERMINED BY THE ENGINEER.
C. SINGLE CONDUITS AND PIPES OR INTERSECTING CONDUITS AND PIPES SHALL NOT OCCUPY MORE THAN AN 1 1/2" OF SLAB THICKNESS AND 1/3 THE OVERALL THICKNESS OF BEAMS IN WHICH THEY ARE EMBEDDED, AND THEY SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER. ANY CONDUIT OR PIPE LARGER SHALL BE LOCATED BELOW THE RESPECTIVE SLAB OR BEAM. THE CONTRACTOR SHALL SUBMIT FOR APPROVAL, A DIAGRAM DEPICTING THE HOME RUNS OF CONDUIT TO ALL PANELS, TYPICAL.
D. IT WILL NOT BE PERMITTED TO CUT, BEND, OR DISPLACE THE REINFORCING STEEL FROM ITS PROPER LOCATION.
E. COORDINATION MUST BE MADE BY THE CONTRACTOR AT HIS EXPENSE TO FOLLOW THE ABOVE GUIDELINES.
13. STRUCTURAL STEEL:
A. A.I.S.C. SPECIFICATIONS; STEEL CONSTRUCTION MANUAL 14TH EDITION;
ALL WIDE FLANGE SHAPES ASTM A992 GRADE 50, ALL MISCELLANEOUS SHAPES ASTM A36, HSS SHAPES ASTM A500 GRADE B Fy=46 ksi, STEEL PIPE ASTM A53 GRADE B Fy=35 ksi; HIGH STRENGTH BOLTS A325, 3/4" > MIN. EXCEPT AS NOTED OTHERWISE; ANCHOR RODS AND BOLTS ASTM F1554 GRADE 36 EXCEPT AS NOTED OTHERWISE; AUTOMATICALLY END WELDED SHEAR CONNECTORS (A.E.W.S.C.) AS E-70 ELECTRODES; SPEEDS: V_{ult} = 144 MPH, V_{asd} = 108 MPH
B. ALL BEAM CONNECTIONS SHALL BE A.I.S.C. STANDARD FRAMED CONNECTIONS. SHOP CONNECTIONS SHALL BE WELDED. FIELD CONNECTIONS SHALL BE BOLTED, EXCEPT CONNECTIONS TO EXISTING CONSTRUCTION SHALL BE WELDED. CONNECTIONS NOT SCHEDULED OR DETAILED OTHERWISE TO BE DESIGNED FOR 1/2 UNIFORM LOAD BEAM CAPACITY FOR PROPER BEAM SPAN AND 2/3 UNIFORM LOAD BEAM CAPACITY FOR PROPER BEAM SPAN FOR COMPOSITE BEAMS.
C. ALL STRUCTURAL STEEL ITEMS AND RESPECTIVE ANCHORS AND FASTENERS PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123. PROVIDE VENT HOLES AS REQUIRED. TOUCH UP ALL DAMAGED COATING WITH STICK GALVANIZING. SEE ARCHITECT FOR PAINTING.
14. COLD FORMED METAL FRAMING:
MEMBERS DESIGNATED SUCH AS 600S162-54, ETC. ARE COLD FORMED METAL FRAMING MEMBERS AS MANUFACTURED BY CLARK STEEL FRAMING SYSTEMS OR APPROVED EQUIV. INSTALL, CONNECT, PROVIDE BRIDGING ETC. IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. ALL MATERIAL 18 GAGE OR THINNER TO BE 33 KSI. ALL MATERIAL GREATER THAN 18 GAGE SHALL BE 50 KSI.
15. METAL FLOOR DECK (NON-COMPOSITE):
0.6C26 GALVANIZED STEEL NON-COMPOSITE TYPE DECK BY VULCRAFT OR APPROVED EQUIV. DECK TO BE CONTINUOUS OVER 3 OR MORE SPANS.
A. DECK PROPERTIES:
1. DEPTH: 9/16"
2. GAGE: 26
3. MIN. SECTION MODULUS (POSITIVE): 0.043 IN³ PER FT.
4. MIN. SECTION MODULUS (NEGATIVE): 0.043 IN³ PER FT.
5. MIN. MOMENT OF INERTIA (POSITIVE): 0.013 IN⁴ PER FT.
6. MIN. MOMENT OF INERTIA (NEGATIVE): 0.015 IN⁴ PER FT.
7. DESIGN THICKNESS: 0.0179"
8. ALLOWABLE STRESS: 60 KSI
B. DECK FASTENING PATTERN:
1. SUPPORT FASTENERS: #12 TEK SCREW
2. SUPPORT FASTENER PATTERN: 30/4
3. SIDELAP FASTENERS: #10 TEK SCREW @ 8" O.C.
16. METAL FLOOR DECK (COMPOSITE):
1.5V20 GALVANIZED STEEL COMPOSITE TYPE DECK BY VULCRAFT OR APPROVED EQUIV. CONTINUOUS OVER 3 OR MORE SPANS.
A. DECK PROPERTIES:
1. DEPTH: 1 1/2"
2. GAGE: 20
3. MIN. SECTION MODULUS (POSITIVE): 0.224 IN³ PER FT.
4. MIN. SECTION MODULUS (NEGATIVE): 0.221 IN³ PER FT.
5. MIN. MOMENT OF INERTIA (POSITIVE): 0.186 IN⁴ PER FT.
6. MIN. MOMENT OF INERTIA (NEGATIVE): 0.222 IN⁴ PER FT.
7. DESIGN THICKNESS: 0.0358"
8. ALLOWABLE STRESS: 50 KSI
17. WELDING PRECAUTIONS:
A. FIRE PREVENTION: AT ALL TIMES DURING WELDING AND TORCH CUTTING IN THE EXISTING BUILDING THE CONTRACTOR SHALL HAVE ADEQUATE FIRE FIGHTING EQUIPMENT READY FOR USE AT THE LOCATION OF THE WELDING. THE WELDING AND TORCH CUTTING PROCESS SHALL BE CLOSELY OBSERVED AND MONITORED BY THE CONTRACTOR SO AS TO PREVENT THE IGNITION OF ANY COMBUSTIBLES IN THE CONSTRUCTION AREA. OBSERVATIONS SHALL CONTINUE FOR AT LEAST 4 HOURS AFTER WELDING IN A PARTICULAR AREA HAS CEASED.
B. PROVIDE ADEQUATE VENTILATION AS TO TOTALLY EXHAUST WELDING FUMES OUT OF THE CONSTRUCTION AREA SO AS NOT TO CONTAMINATE ANY PORTION OF THE EXISTING BUILDING.
18. POST INSTALLED ANCHORS TO HARDENED CONCRETE:
A. IN THE INSTALLATION OF ANCHORS, CARE SHALL BE EXERCISED SO AS NOT TO NICK OR CUT EXISTING REINFORCING, CONDUIT, ETC. SEE GENERAL NOTE 4 FOR PROCEDURE FOR DRILLING HOLES IN EXISTING CONCRETE.
B. ANCHOR TYPES SHALL BE AS FOLLOWS:
1. WEDGE TYPE ANCHORS: "Kwik Bolt 3" BOLTS AS MANUFACTURED BY HILTI FASTENING SYSTEMS OR AN APPROVED EQUIV.
2. ADHESIVE TYPE ANCHORS (CONCRETE):
"HAS-E" RODS AND HIT-HY 200 MAX ADHESIVE SYSTEM AS MANUFACTURED BY HILTI FASTENING SYSTEMS OR AN APPROVED EQUIV.
3. ADHESIVE TYPE ANCHORS (GROUT FILLED & HOLLOW CMU):
"HAS-E" RODS AND HIT-HY TO ADHESIVE SYSTEM AS MANUFACTURED BY HILTI FASTENING SYSTEMS OR AN APPROVED EQUIV. ADDITIONALLY, AT HOLLOW CMU CONDITIONS, PROVIDE HILTI HIT-SC SCREENS.
C. SUBMIT PRODUCT DATA AND I.C.C. TEST REPORTS FOR REVIEW AND APPROVAL. REFER TO SPECIFICATIONS FOR SUBMITTAL REQUIREMENTS.
19. OTHER WORK:
COORDINATE ALL OTHER WORK WITH STRUCTURAL. UNLESS DETAILED OR SPECIFIED, THE ARCHITECT IS TO APPROVE ALL OPENINGS, SLEEVES, EMBEDDED ITEMS ETC. INVOLVED IN STRUCTURAL WORK PRIOR TO THEIR BEING SET. DO NOT CUT OR DRILL HOLES IN STRUCTURAL MEMBERS WITHOUT THE APPROVAL OF THE ARCHITECT. ALL SUCH ITEMS SHALL NOT IMPAIR THE STRUCTURAL INTEGRITY OF THE MEMBER AS DETERMINED BY THE ENGINEER OF RECORD.
20. CONCRETE MASONRY UNIT CONSTRUCTION:
A. ALL MASONRY CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 530/530.1-11 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES.
B. ALL MASONRY UNITS SHALL BE IN ACCORDANCE WITH ASTM C 90. ASSEMBLED CONCRETE MASONRY SHALL ATTAIN AN ULTIMATE NET AREA COMPRESSIVE STRENGTH (f_m) OF 1,500 PSI.
C. ALL MORTAR SHALL BE 1,800 PSI, TYPE S IN CONFORMANCE WITH ASTM C 270.
D. ALL REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ASTM A615, GRADE 60. SEE DETAILS FOR HORIZONTAL AND VERTICAL REINFORCING REQUIREMENTS. VERTICAL REINFORCING SHALL BE POSITIVELY SECURED IN PLACE AT 4'-0" MAX.
E. THE MASONRY CONTRACTOR SHALL BUILD AND GROUT THE WALLS IN 4'-0" LIFTS (MAX.), VIBRATING THE GROUT IMMEDIATELY AFTER PLACEMENT.
F. ALL REINFORCED CELLS SHALL BE FULLY GROUTED FROM TOP TO BOTTOM. GROUT SHALL BE 3,000 PSI FINE GROUT IN ACCORDANCE WITH ASTM C476.
G. PROVIDE REBAR DOWELS OF SAME SIZE AND SPACING AS VERTICAL REINFORCING FROM WALLS, SLABS, BEAMS OR FOOTINGS BELOW. DOWELS INTO FOOTINGS SHALL HAVE STANDARD ACI HOOKS.
H. VERTICAL BARS SHALL BE FULL HEIGHT BETWEEN SUPPORTS. AT THE CONTRACTOR'S OPTION, CMU H-BLOCKS, OF EQUAL COMPRESSIVE STRENGTH MAY BE USED. AT CONTRACTOR'S OPTION, IN LIEU OF FULL-HEIGHT REINFORCING, MECHANICAL COUPLERS WHICH DEVELOP 125% OF ASTM SPECIFIED TENSILE STRENGTH, MAY BE USED.
I. VERTICAL CELLS TO BE FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS, VERTICAL CELL MEASURING NOT LESS THAN 2 INCHES BY 3 INCHES.
J. CONTRACTOR SHALL PROVIDE FULL SHOP DRAWINGS TO INCLUDE ALL REINFORCING, MECHANICAL CONNECTORS, SPECIALTY ITEMS, MORTAR AND GROUT MIX, ETC. PRIOR TO PLACING OF ANY CONSTRUCTION.
K. AN INDEPENDENT TESTING LAB, AS HIRED BY THE OWNER, SHALL PERFORM PERIODIC ON-SITE OBSERVATION TO OBSERVE MORTAR AND GROUT MIXING AND PLACEMENT, BAR PLACEMENT, COUPLER INSTALLATION, BOND BEAM CONSTRUCTION, ETC.
21. DESIGN LOADS AND OTHER PERTINENT DESIGN INFORMATION:
A. BUILDING CODE: INTERNATIONAL BUILDING CODE 2012 / ASCE 7-10
B. FLOOR LIVE LOAD
1. 40 PSF
C. ROOF LIVE LOAD
1. 20 PSF
D. ROOF SNOW LOAD
1. GROUND SNOW LOAD (P_g): 0 PSF
E. WIND LOAD
1. WIND SPEEDS: V_{ult} = 144 MPH, V_{asd} = 108 MPH
2. RISK CATEGORY: II
3. WIND EXPOSURE CATEGORY: B
4. INTERNAL PRESSURE COEFFICIENT: ±0.18
5. EDGE WIDTH "a": 10 FEET
6. COMPONENTS AND CLADDING PRESSURES:

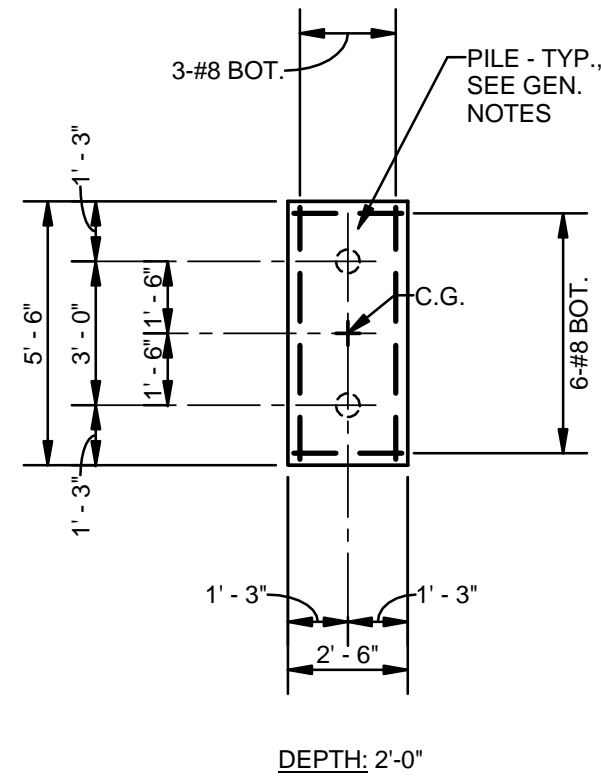
COMPONENTS AND CLADDING DESIGN PRESSURES (PSF)												
ZONE	1	2	3	4	5							
EWA (FT ²)	1	2	3	4	5	6	7	8	9	10	11	12
≤10	-43	18	-72	18	-109	18	-42	39	-52	39		
20	-42	16	-65	16	-91	16	-40	37	-49	37		
50	-41	15	-54	15	-65	15	-38	36	-44	36		
100	-40	14	-47	14	-47	14	-36	34	-41	34		
200	-40	14	-47	14	-47	14	-35	31	-37	31		
≥500	-40	14	-47	14	-47	14	-32	29	-32	29		

NOTES:
1.) EWA IS THE EFFECTIVE WIND AREA OF A STRUCTURAL COMPONENT.
2.) FOR ZONE DEFINITIONS, SEE ASCE 7-10 FIGURES 30.4-1 AND 30.4-2A.
3.) PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING FORWARD AND AWAY FROM THE SURFACES, RESPECTIVELY.



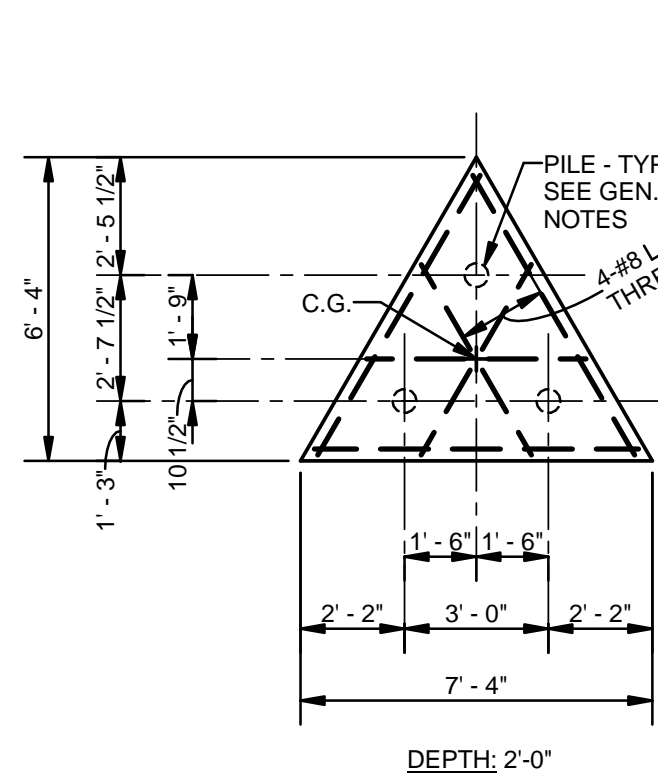
DETAIL-ELEVATOR PILE CAP (4PB)

5
S001 1/4" = 1'-0"



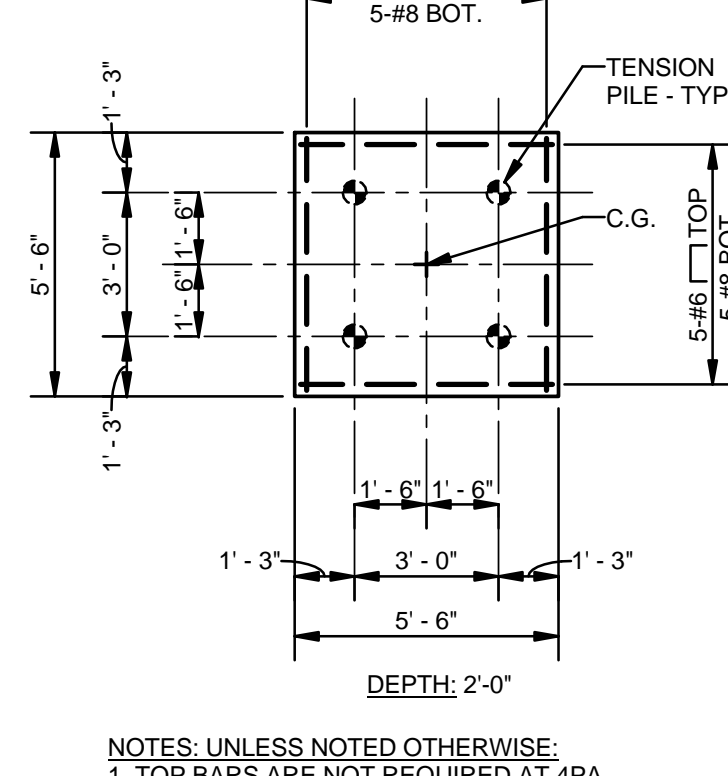
DETAIL-2 PILE CAP (2PA)

S001 1/4" = 1'-0"



DETAIL-3 PILE CAP (3PA)

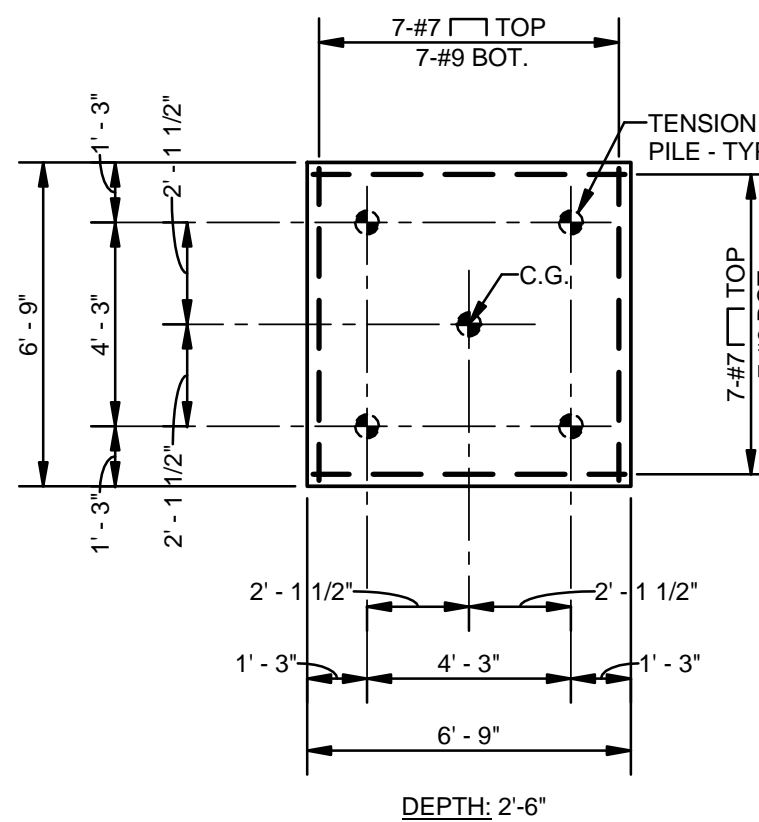
S001 1/4" = 1'-0"



DETAIL-4 PILE CAP (4PA)(4PT)

S001 1/4" = 1'-0"

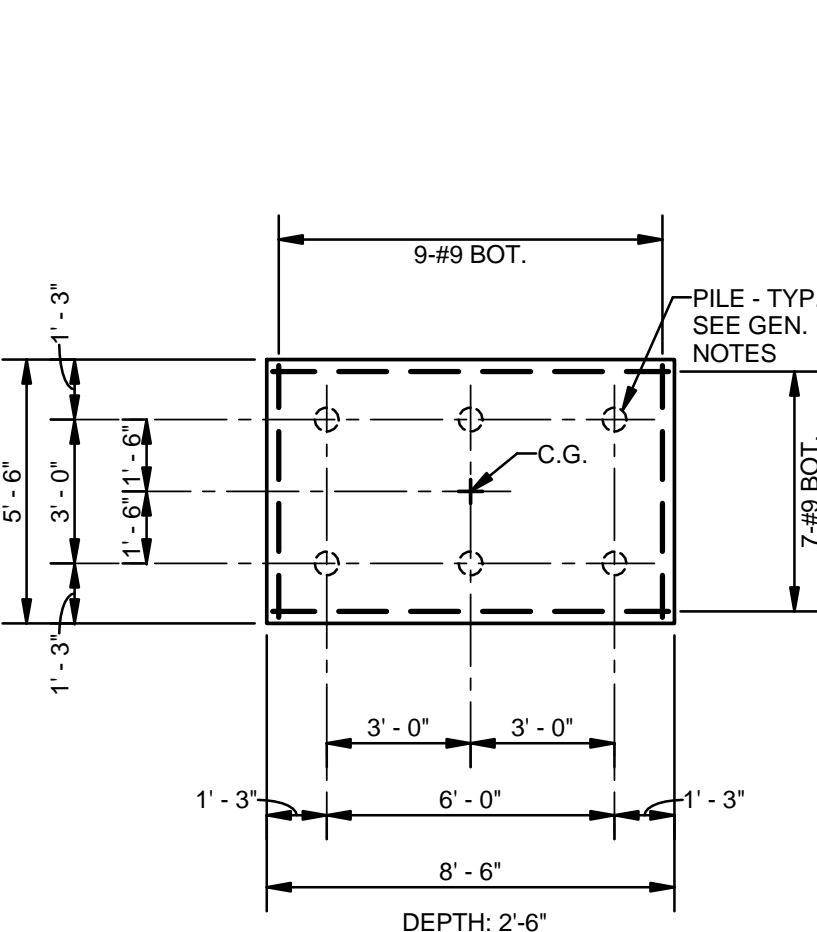
NOTES: UNLESS NOTED OTHERWISE:
1. TOP BARS ARE NOT REQUIRED AT 4PA.
2. TENSION PILES ARE NOT REQUIRED AT 4PA.



NOTES: UNLESS NOTED OTHERWISE:
1. TOP BARS ARE NOT REQUIRED AT 5PA.
2. TENSION PILES ARE NOT REQUIRED AT 5PA.

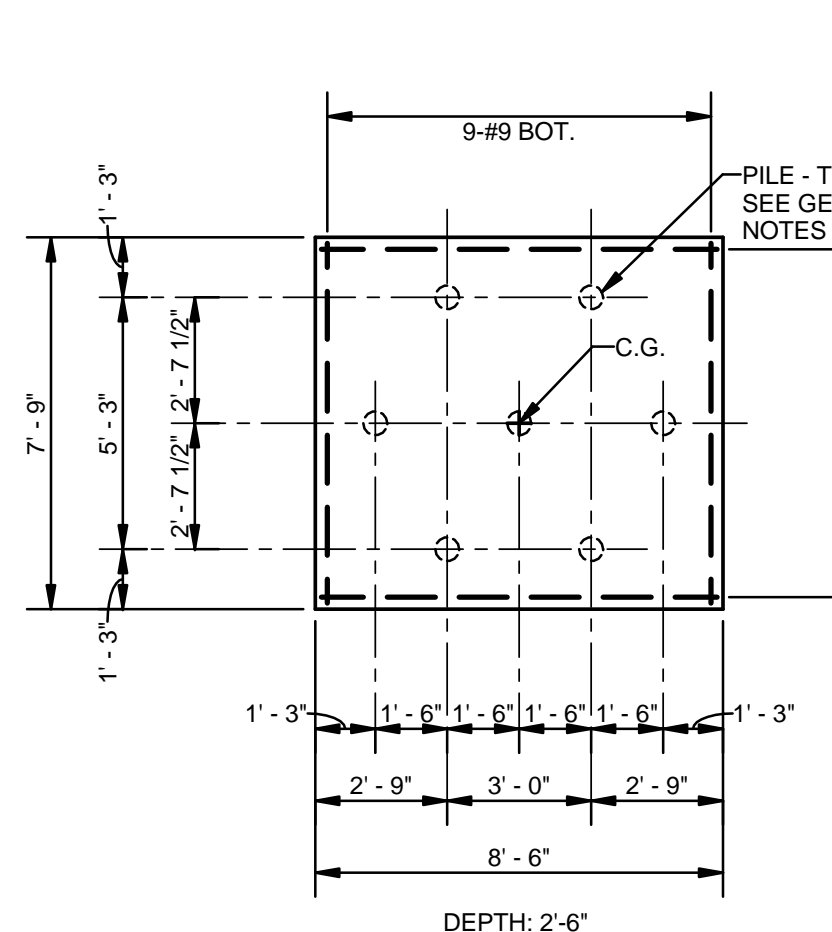
DETAIL-5 PILE CAP (5PA)(5PT)

S001 1/4" = 1'-0"



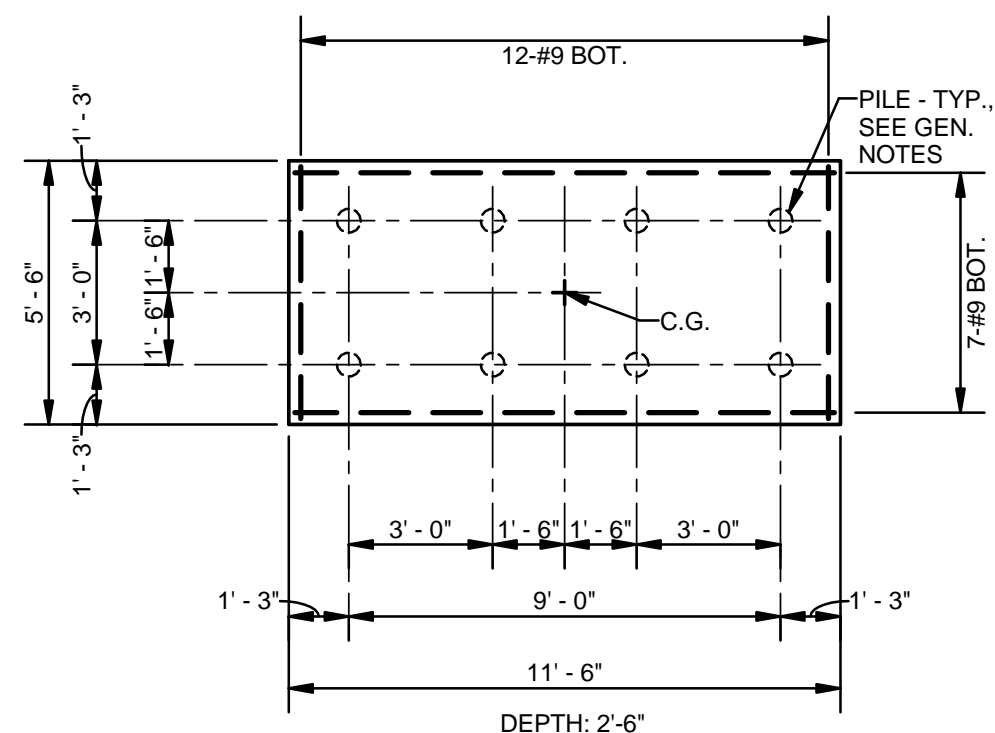
DETAIL-6 PILE CAP (6PA)

S001 1/4" = 1'-0"



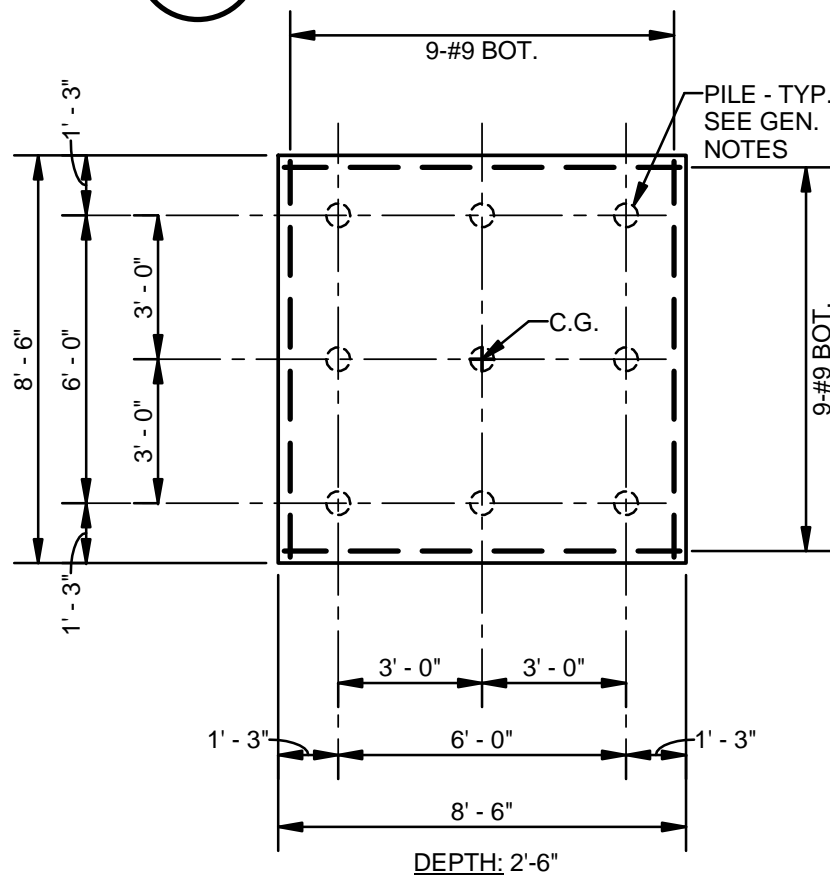
DETAIL-7 PILE CAP (7PA)

S001 1/4" = 1'-0"



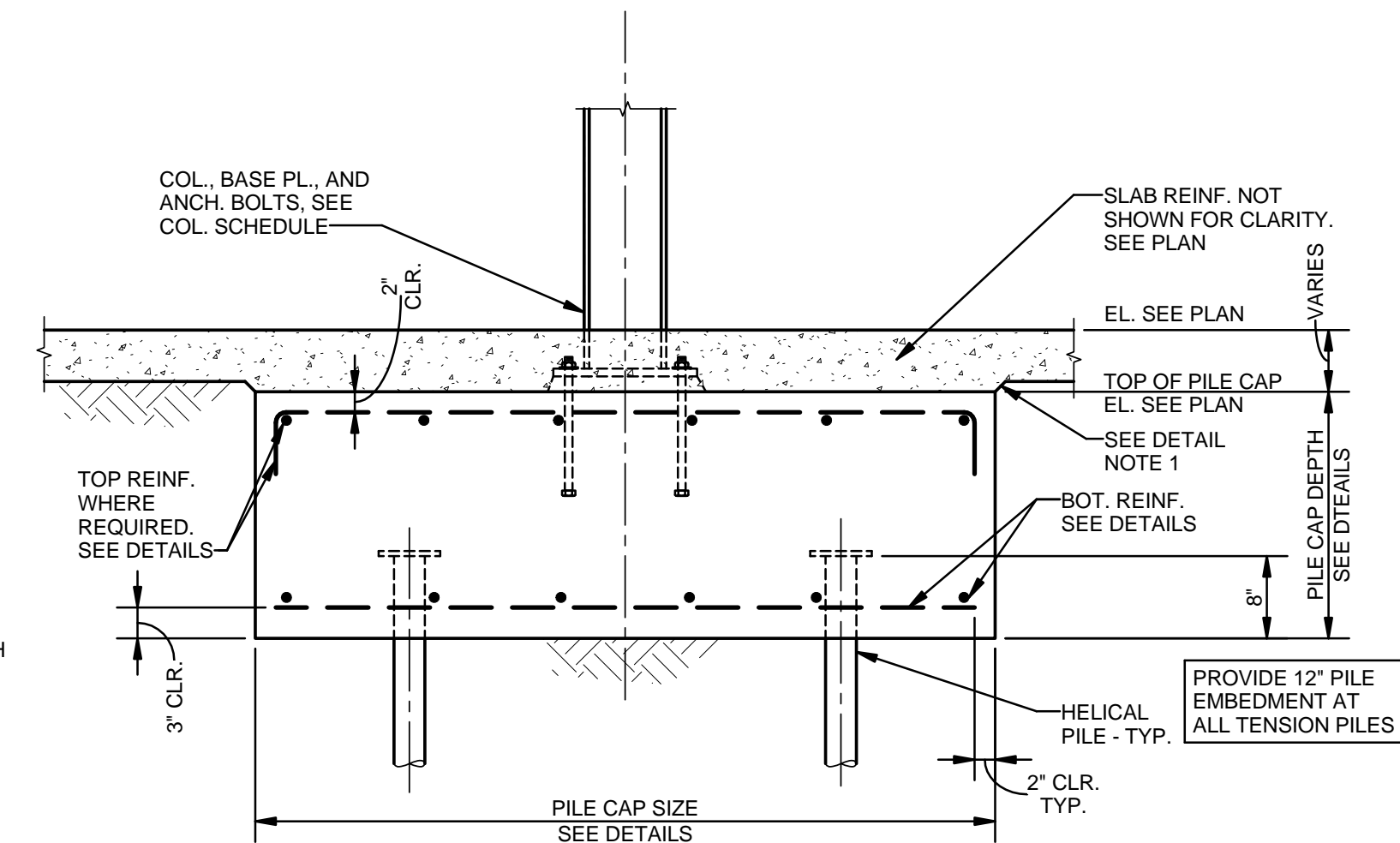
DETAIL-8 PILE CAP (8PA)

S001 1/4" = 1'-0"



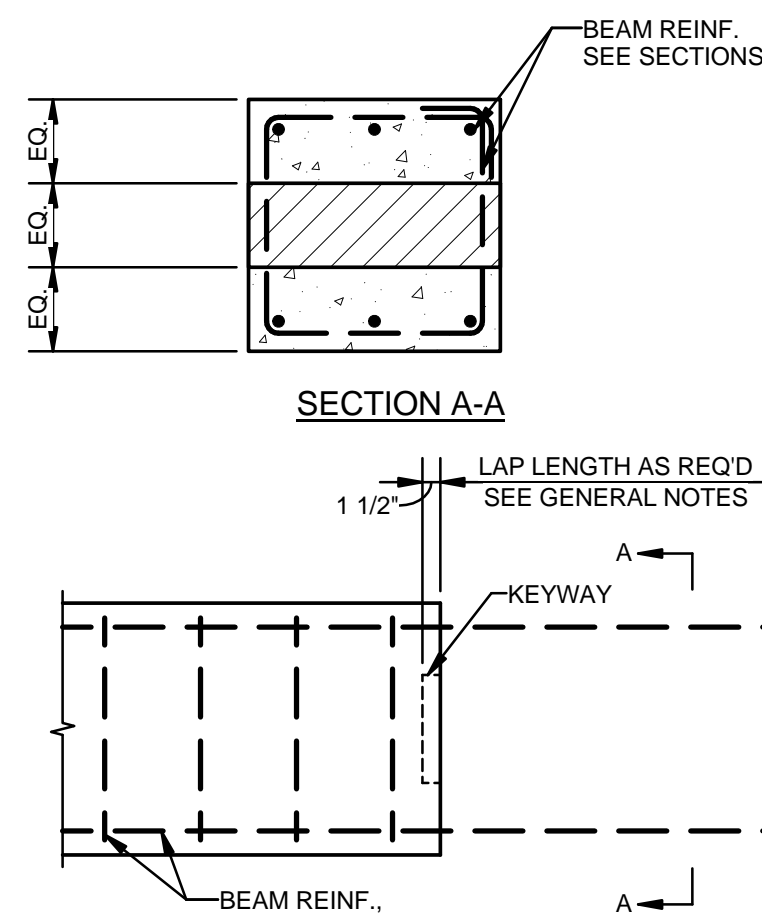
DETAIL-9 PILE CAP (9PA)

S001 1/4" = 1'-0"



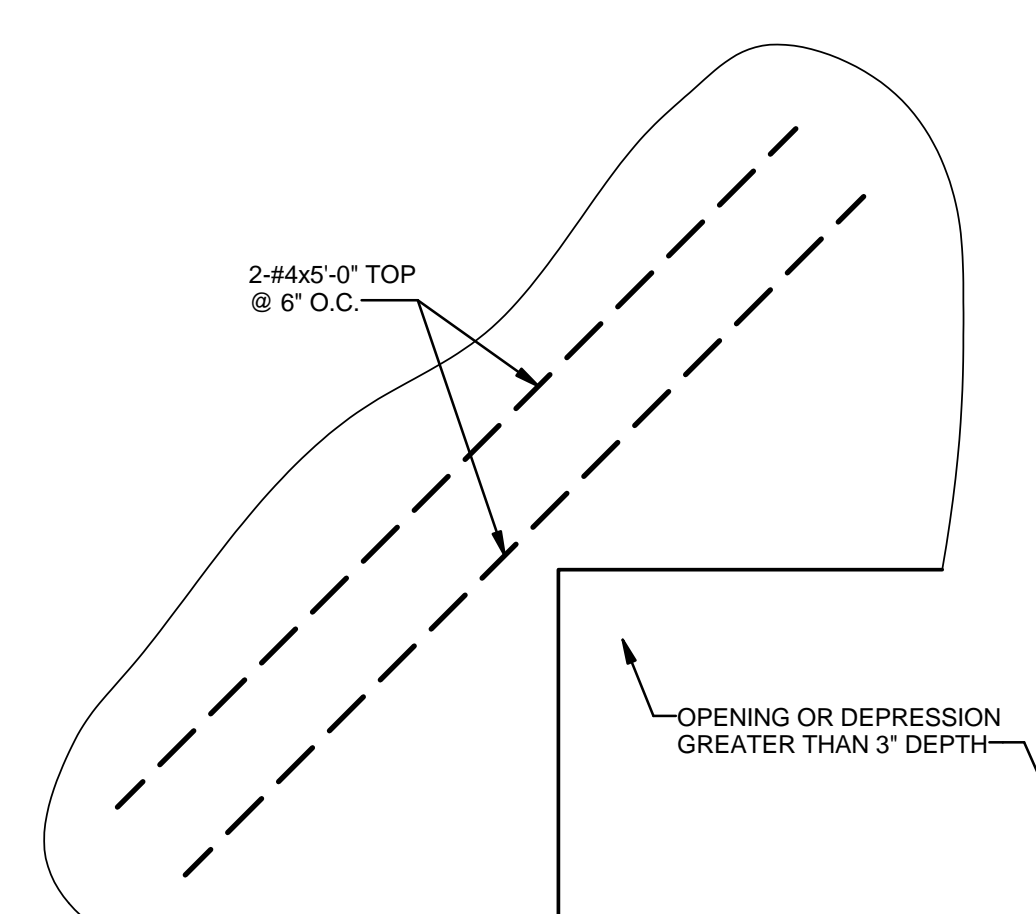
DETAIL-TYPICAL PILE CAP

S001 3/4" = 1'-0"



DETAIL-TYPICAL CONCRETE BEAM BULKHEAD

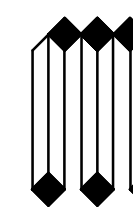
S001 3/4" = 1'-0"



DETAIL-TYPICAL REENTRANT SLAB CORNER

S001 1" = 1'-0"

PROGRESS PRINT
12/09/16



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REVISION HISTORY									
NO.	DATE	DESCRIPTION	DESIGN DEVELOPMENT	FOUNDATION PERMIT SET	PROGRESS SET	PERMIT SET			
1	06/20/16	SCHEMATIC DESIGN							
2	07/14/16	DESIGN DEVELOPMENT							
3	08/26/16	FOUNDATION PERMIT SET							
4	09/16/16	PROGRESS SET							
5	09/27/16	PERMIT SET							

GENERAL NOTES & PILE CAP DETAILS

PROJECT#: 1435
PHASE: DD
DRAFTER: HSB
CHECKER: HSB
SCALE: AS NOTED
ISSUED: 08-26-16

SHEET#:

S001